

MEDICAL ELEVATED STORAGE TANK (EST) OVERCOAT PROJECT SAWS Job No. 19-0128 SAWS Solicitation No. B-19-014-JAM

ADDENDUM NO. 2 August 23, 2019

To Respondent of Record:

This addendum, applicable to work referenced above, is an amendment to the bidding documents and as such will be a part of and included in the Contract Documents. Acknowledge receipt of this addendum by entering the addendum number and issue date in the space provided in the submitted price proposal.

RESPONSES TO SUBMITTED QUESTIONS

1. No questions were submitted for this project during the required timeframe noted in the Invitation to Respondents.

MODIFICATIONS TO THE SPECIFICATIONS

- 1. Add "05120 Structural Steel Framing" and remove "05500 Metal Fabrications" from the Project Table of Contents
- 2. Add Specification 05120 Structural Steel Framing in its entirety and remove 05500 Metal Fabrications in its entirety.
- 3. Remove Specification 09871 Exterior Coating System for Steel Storage Tanks in its entirely and replace with revised version included in this Addendum. The allowable products from PPG were updated.

MODIFICATIONS TO THE PLANS

1. Removal all Paul J. Ford Structural Plans in their entirety and replace them with Tetra Tech "S-001 Structural General Notes", "S-101 Tank Roof Plan", "S-501 Structural Typical Details" Plans.

This Addendum, is thirteen (21) pages in its entirety.

Attachments: 05120 – Structural Steel Framing (10 pages)

- 09871 Exterior Coating System for Steel Storage Tanks (6 pages)
- S-001 Structural General Notes
- S-101 Tank Roof Plan
- S-501 Structural Typical Details



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PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. Section Includes:
 - 1. Structural steel.

1.3 DEFINITIONS

A. Structural Steel: Elements of the structural frame indicated on Drawings and as described in AISC 303, "Code of Standard Practice for Steel Buildings and Bridges."

1.4 COORDINATION

- A. Coordinate selection of shop primers with topcoats to be applied over them. Comply with paint and coating manufacturers' written recommendations to ensure that shop primers and topcoats are compatible with one another.
- B. Coordinate installation of anchorage items to be embedded in or attached to other construction without delaying the Work. Provide setting diagrams, sheet metal templates, instructions, and directions for installation.

1.5 ACTION SUBMITTALS

- A. Product Data: For each type of product.
- B. Shop Drawings: Show fabrication of structural-steel components.
 - 1. Include details of cuts, connections, splices, camber, holes, and other pertinent data.
 - 2. Include embedment Drawings.
 - 3. Indicate welds by standard AWS symbols, distinguishing between shop and field welds, and show size, length, and type of each weld. Show backing bars that are to be removed and supplemental fillet welds where backing bars are to remain.
 - 4. Indicate type, size, and length of bolts, distinguishing between shop and field bolts. Identify pretensioned and slip-critical, high-strength bolted connections.
 - 5. Indicate locations and dimensions of protected zones.

- C. Welding Procedure Specifications (WPSs) and Procedure Qualification Records (PQRs): Provide according to AWS D1.1/D1.1M, "Structural Welding Code - Steel," for each welded joint whether prequalified or qualified by testing, including the following:
 - 1. Power source (constant current or constant voltage).
 - 2. Electrode manufacturer and trade name, for demand critical welds.

1.6 INFORMATIONAL SUBMITTALS

- A. Welding certificates.
- B. Paint Compatibility Certificates: From manufacturers of topcoats applied over shop primers, certifying that shop primers are compatible with topcoats.
- C. Mill test reports for structural steel, including chemical and physical properties.
- D. Product Test Reports: For the following:
 - 1. Bolts, nuts, and washers including mechanical properties and chemical analysis.
 - 2. Direct-tension indicators.
 - 3. Tension-control, high-strength, bolt-nut-washer assemblies.
 - 4. Shop primers.
- E. Survey of existing conditions.
- F. Source quality-control reports.
- G. Field quality-control and special inspection reports.

1.7 QUALITY ASSURANCE

- A. Fabricator Qualifications: A qualified fabricator that participates in the AISC Quality Certification Program and is designated an AISC-Certified Plant, Category STD, or is accredited by the IAS Fabricator Inspection Program for Structural Steel (AC 172).
- B. Welding Qualifications: Qualify procedures and personnel according to AWS D1.1/D1.1M, "Structural Welding Code Steel."
 - 1. Welders and welding operators performing work on bottom-flange, demand-critical welds shall pass the supplemental welder qualification testing, as required by AWS D1.8/D1.8M. FCAW-S and FCAW-G shall be considered separate processes for welding personnel qualification.
- C. Comply with applicable provisions of the following specifications and documents:
 - 1. AISC 303.
 - 2. AISC 341 and AISC 341s1.
 - 3. AISC 360.

SAN ANTONIO WATER SYSTEM STRUCTURAL STEEL FRAMING

4. RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts."

1.8 DELIVERY, STORAGE, AND HANDLING

- A. Store materials to permit easy access for inspection and identification. Keep steel members off ground and spaced by using pallets, dunnage, or other supports and spacers. Protect steel members and packaged materials from corrosion and deterioration.
 - 1. Do not store materials on structure in a manner that might cause distortion, damage, or overload to members or supporting structures. Repair or replace damaged materials or structures as directed.
- B. Store fasteners in a protected place in sealed containers with manufacturer's labels intact.
 - 1. Fasteners may be repackaged provided Owner's testing and inspecting agency observes repackaging and seals containers.
 - 2. Clean and relubricate bolts and nuts that become dry or rusty before use.
 - 3. Comply with manufacturers' written recommendations for cleaning and lubricating ASTM F 1852 fasteners and for retesting fasteners after lubrication.

PART 2 - PRODUCTS

2.1 STRUCTURAL-STEEL MATERIALS

- A. W-Shapes: ASTM A 992/A 992M.
- B. Channels, Angles, S-Shapes: ASTM A 36/A 36M.
- C. Plate and Bar: ASTM A 36/A 36M.
- D. Steel Pipe: ASTM A 53/A 53M, Type E or Type S, Grade B.
 - 1. Weight Class: as indicated.
 - 2. Finish: Black except where indicated to be galvanized.
- E. Welding Electrodes: Comply with AWS requirements.
- F. Guard Posts: Fabricate guard posts from pipe as indicated on plans. Cap posts with 1/4-inch minimum thickness steel base plate. See Standard Detail on Drawings.

2.2 BOLTS, CONNECTORS, AND ANCHORS

A. High-Strength Bolts, Nuts, and Washers: ASTM A 325, Type 1, heavy-hex steel structural bolts; ASTM A 563, Grade C, heavy-hex carbon-steel nuts; and ASTM F 436, Type 1, hardened carbon-steel washers; all with plain finish.

- 1. Direct-Tension Indicators: ASTM F 959, Type 325, compressible-washer type with plain finish.
- B. Zinc-Coated High-Strength Bolts, Nuts, and Washers: ASTM A 325, Type 1, heavy-hex steel structural bolts; ASTM A 563, Grade DH heavy-hex carbon-steel nuts; and ASTM F 436, Type 1, hardened carbon-steel washers.
 - 1. Finish: Hot-dip or mechanically deposited zinc coating.
 - 2. Direct-Tension Indicators: ASTM F 959, Type 325, compressible-washer type with mechanically deposited zinc coating, baked epoxy-coated finish.
 - 3. Location: exterior applications or where supporting members are galvanized.
- C. Tension-Control, High-Strength Bolt-Nut-Washer Assemblies: ASTM F 1852, Type 1, heavyhex head assemblies consisting of steel structural bolts with splined ends, heavy-hex carbonsteel nuts, and hardened carbon-steel washers.
 - 1. Finish: Plain.
 - 2. Location: Interior applications or where supporting members are painted.
- D. Threaded Rods: ASTM A 572/A 572M, Grade 50.
 - 1. Nuts: ASTM A 563 [heavy-] hex carbon steel.
 - 2. Washers: ASTM A 36/A 36M carbon steel.
 - 3. Finish: Hot-dip zinc coating, ASTM A 153/A 153M, Class C, for exterior applications or where columns are galvanized. Use plain for painted columns.

2.3 PRIMER

A. Primer: Comply with Section 09871 "Exterior Coating," Section 09872 "Interior Coating," and Section 09900 "General Specification for Coating Systems."

2.4 FABRICATION

- A. Structural Steel: Fabricate and assemble in shop to greatest extent possible. Fabricate according to AISC 303, "Code of Standard Practice for Steel Buildings and Bridges," and to AISC 360.
 - 1. Camber structural-steel members where indicated.
 - 2. Fabricate beams with rolling camber up.
 - 3. Identify high-strength structural steel according to ASTM A 6/A 6M and maintain markings until structural steel has been erected.
 - 4. Mark and match-mark materials for field assembly.
 - 5. Complete structural-steel assemblies, including welding of units, before starting shoppriming operations.
- B. Thermal Cutting: Perform thermal cutting by machine to greatest extent possible.
 - 1. Plane thermally cut edges to be welded to comply with requirements in AWS D1.1/D1.1M.

- C. Bolt Holes: Cut, drill,or punch standard bolt holes perpendicular to metal surfaces.
- D. Finishing: Accurately finish ends of columns and other members transmitting bearing loads.
- E. Cleaning: Clean and prepare steel surfaces that are to remain unpainted according to SSPC-SP 1, "Solvent Cleaning."
- F. Shear Connectors: Prepare steel surfaces as recommended by manufacturer of shear connectors. Use automatic end welding of headed-stud shear connectors according to AWS D1.1/D1.1M and manufacturer's written instructions.
- G. Holes: Provide holes required for securing other work to structural steel and for other work to pass through steel members.
 - 1. Cut, drill, or punch holes perpendicular to steel surfaces. Do not thermally cut bolt holes or enlarge holes by burning.
 - 2. Baseplate Holes: Cut, drill, mechanically thermal cut, or punch holes perpendicular to steel surfaces.
 - 3. Weld threaded nuts to framing and other specialty items indicated to receive other work.

2.5 SHOP CONNECTIONS

- A. High-Strength Bolts: Shop install high-strength bolts according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" for type of bolt and type of joint specified.
 - 1. Joint Type: Snug tightened.
- B. Weld Connections: Comply with AWS D1.1/D1.1M for tolerances, appearances, welding procedure specifications, weld quality, and methods used in correcting welding work.
 - 1. Assemble and weld built-up sections by methods that maintain true alignment of axes without exceeding tolerances in AISC 303 for mill material.

2.6 SHOP PRIMING

- A. Shop prime steel surfaces except the following:
 - 1. Surfaces to be field welded.
 - 2. Surfaces to receive sprayed fire-resistive materials (applied fireproofing).
 - 3. Galvanized surfaces.
 - 4. Surfaces enclosed in interior construction.
- B. Surface Preparation: as indicated by Section 09900 "General Specification for Coating Systems."
- C. Priming: Immediately after surface preparation, apply primer according to manufacturer's written instructions and at rate recommended by SSPC to provide a minimum dry film thickness

of 1.5 mils. Use priming methods that result in full coverage of joints, corners, edges, and exposed surfaces.

- 1. Stripe paint corners, crevices, bolts, welds, and sharp edges.
- 2. Apply two coats of shop paint to surfaces that are inaccessible after assembly or erection. Change color of second coat to distinguish it from first.

2.7 GALVANIZING

- A. Hot-Dip Galvanized Finish: Apply zinc coating by the hot-dip process to structural steel according to ASTM A 123/A 123M.
 - 1. Fill vent and drain holes that are exposed in the finished Work unless they function as weep holes, by plugging with zinc solder and filing off smooth.
 - 2. Galvanize lintels, shelf angles and welded door frames attached to structural-steel frame and located in exterior walls.
 - 3. Galvanize all exterior steel and where indicated on the drawings.

2.8 SOURCE QUALITY CONTROL

- A. Testing Agency: Owner will engage a qualified testing agency to perform shop tests and inspections.
 - 1. Provide testing agency with access to places where structural-steel work is being fabricated or produced to perform tests and inspections.
- B. Bolted Connections: Inspect and test shop-bolted connections according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts."
- C. Welded Connections: Visually inspect shop-welded connections according to AWS D1.1/D1.1M and the following inspection procedures, at testing agency's option:
 - 1. Liquid Penetrant Inspection: ASTM E 165.
 - 2. Magnetic Particle Inspection: ASTM E 709; performed on root pass and on finished weld. Cracks or zones of incomplete fusion or penetration are not accepted.
 - 3. Ultrasonic Inspection: ASTM E 164.
 - 4. Radiographic Inspection: ASTM E 94.
- D. In addition to visual inspection, test and inspect shop-welded shear connectors according to requirements in AWS D1.1/D1.1M for stud welding and as follows:
 - 1. Perform bend tests if visual inspections reveal either a less-than-continuous 360-degree flash or welding repairs to any shear connector.
 - 2. Conduct tests according to requirements in AWS D1.1/D1.1M on additional shear connectors if weld fracture occurs on shear connectors already tested.
- E. Prepare test and inspection reports.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Verify, with certified steel erector present, elevations of concrete- and masonry-bearing surfaces and locations of anchor rods, bearing plates, and other embedments for compliance with requirements.
 - 1. Prepare a certified survey of existing conditions. Include bearing surfaces, anchor rods, bearing plates, and other embedments showing dimensions, locations, angles, and elevations.
- B. Proceed with installation only after unsatisfactory conditions have been corrected.

3.2 PREPARATION

- A. Provide temporary shores, guys, braces, and other supports during erection to keep structural steel secure, plumb, and in alignment against temporary construction loads and loads equal in intensity to design loads. Remove temporary supports when permanent structural steel, connections, and bracing are in place unless otherwise indicated.
 - 1. Do not remove temporary shoring supporting composite deck construction until cast-inplace concrete has attained its design compressive strength.

3.3 ERECTION

- A. Set structural steel accurately in locations and to elevations indicated and according to AISC 303 and AISC 360.
- B. Maintain erection tolerances of structural steel within AISC 303, "Code of Standard Practice for Steel Buildings and Bridges."
- C. Align and adjust various members that form part of complete frame or structure before permanently fastening. Before assembly, clean bearing surfaces and other surfaces that are in permanent contact with members. Perform necessary adjustments to compensate for discrepancies in elevations and alignment.
 - 1. Level and plumb individual members of structure.
 - 2. Make allowances for difference between temperature at time of erection and mean temperature when structure is completed and in service.
- D. Splice members only where indicated.
- E. Do not use thermal cutting during erection unless approved by ENGINEER. Finish thermally cut sections within smoothness limits in AWS D1.1/D1.1M.

- F. Do not enlarge unfair holes in members by burning or using drift pins. Ream holes that must be enlarged to admit bolts.
- G. Shear Connectors: Prepare steel surfaces as recommended by manufacturer of shear connectors. Use automatic end welding of headed-stud shear connectors according to AWS D1.1/D1.1M and manufacturer's written instructions.

3.4 FIELD CONNECTIONS

- A. High-Strength Bolts: Install high-strength bolts according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" for type of bolt and type of joint specified.
 - 1. Joint Type: Snug tightened.
- B. Weld Connections: Comply with AWS D1.1/D1.1M and AWS D1.8/D1.8M for tolerances, appearances, welding procedure specifications, weld quality, and methods used in correcting welding work.
 - 1. Comply with AISC 303 and AISC 360 for bearing, alignment, adequacy of temporary connections, and removal of paint on surfaces adjacent to field welds.
 - 2. Remove backing bars or runoff tabs, back gouge, and grind steel smooth.
 - 3. Assemble and weld built-up sections by methods that maintain true alignment of axes without exceeding tolerances in AISC 303, "Code of Standard Practice for Steel Buildings and Bridges," for mill material.

3.5 FIELD QUALITY CONTROL

- A. Special Inspections: Owner will engage a qualified special inspector to perform the following special inspections:
 - 1. Verify structural-steel materials and inspect steel frame joint details.
 - 2. Verify weld materials and inspect welds.
 - 3. Verify connection materials and inspect high-strength bolted connections.
- B. Bolted Connections: Inspect and test bolted connections according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts."
- C. Welded Connections: Visually inspect field welds according to AWS D1.1/D1.1M.
 - 1. In addition to visual inspection, test and inspect field welds according to AWS D1.1/D1.1M and the following inspection procedures, at testing agency's option:
 - a. Liquid Penetrant Inspection: ASTM E 165.
 - b. Magnetic Particle Inspection: ASTM E 709; performed on root pass and on finished weld. Cracks or zones of incomplete fusion or penetration are not accepted.
 - c. Ultrasonic Inspection: ASTM E 164.

- d. Radiographic Inspection: ASTM E 94.
- D. In addition to visual inspection, test and inspect field-welded shear connectors according to requirements in AWS D1.1/D1.1M for stud welding and as follows:
 - 1. Perform bend tests if visual inspections reveal either a less-than-continuous 360-degree flash or welding repairs to any shear connector.
 - 2. Conduct tests according to requirements in AWS D1.1/D1.1M on additional shear connectors if weld fracture occurs on shear connectors already tested.

3.6 REPAIRS AND PROTECTION

- A. Galvanized Surfaces: Clean areas where galvanizing is damaged or missing and repair galvanizing to comply with ASTM A 780/A 780M.
- B. Touchup Painting: Immediately after erection, clean exposed areas where primer is damaged or missing and paint with the same material as used for shop painting to comply with SSPC-PA 1 for touching up shop-painted surfaces.
 - 1. Clean and prepare surfaces by SSPC-SP 2 hand-tool cleaning or SSPC-SP 3 power-tool cleaning.
- C. Touchup Painting: Cleaning and touchup painting are specified in Section 09900 "General Specification for Coating Systems."

END OF SECTION 05120

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SAN ANTONIO WATER SYSTEM STRUCTURAL STEEL FRAMING

05120 - 10 (Addendum 2)

SECTION 09871

EXTERIOR COATING SYSTEM FOR STEEL STORAGE TANKS

PART 1 GENERAL

1.01 SECTION INCLUDES:

- A. Coating systems for tank exterior surfaces including but not limited to:
 - 1. the reservoir/container;
 - 2. all inlet/outlet and overflow piping;
 - 3. tank riser pipe, all tank support legs and all tank bracing;
 - 4. vents, manholes, ladders, platforms;
 - 5. all metal fabrications including roof equipment rails and support angles; and
 - 6. bolts, threads, nuts, pins, brackets, seams, corners, knife edges, welds, etc.

1.02 RELATED SECTIONS

A. Section 09900 – General Specifications for Coating Systems

1.03 SUBMITTALS

- A. Provide the following in conformance with applicable requirements contained in Specification 01300 Submittals.
 - 1. Coating system to be applied on the tank interior wet surfaces.
 - 2. Applicator Certifications: Submit supervisor and applicator certifications from the coating manufacturer for the coating system that is to be applied.

PART 2 PRODUCTS

2.01 COATING SCHEDULE

- A. Acceptable coating manufacturers and specifications for the exterior surfaces of steel water storage tanks, exterior piping and valves and exposed exterior concrete surfaces follow; however, the CONTRACTOR is advised that all Manufacturers presented below must submit and certify that the coatings furnished are in compliance with these Specifications and the Manufacturer's recommendations.
- B. Tank Exterior: Coating System to be of the same Manufacturer of all other coating products used on this project and shall be as follows:

| Coating System | Coat | Product | DFT (Min- imum) | Color |
|---------------------|------------------------|--|--------------------|------------|
| | Epoxy Mastic Spot Coat | Carbomastic 15 | 5-6 mils | Mill White |
| | Sealer Coat | Rustbond | 1-2 mils | Beige |
| Carboline | Finish Coat (Overcoat) | Carboxane 2000 | 5 mils min | Mill White |
| | Min DFT for System | | 11 mils | |
| | Epoxy Mastic Spot Coat | Epoxy Mastic Aluminum II | 5-6 mils | Mill White |
| Sherwin Williams | Sealer Coat | Macropoxy 920 PrePrime Rust Penetrating Epoxy | 1-2 mils | Beige |
| | Finish Coat (Overcoat) | Sher-Loxane 800 | 5 mils min | Mill White |
| | Min DFT for Sy | 11 mils | | |
| | Epoxy Mastic Spot Coat | Amercoat 278 Amercoat 2/400 | 5-6 mils | Mill White |
| | Sealer Coat | Amerlock Sealer | 1-2 mils | Beige |
| PPG | Finish Coat (Overcoat) | Carboxane 2000 PSX 700 | 5 mils min | Mill White |
| | Min DFT for Sy | 10 mils | | |

Thinners: Only thinners recommended and furnished by the chosen coating Manufacturer shall be used to thin the paint products.

C. Exterior Equipment Railing and Angle Irons: Coating System to be of the same Manufacturer of all other coating products used on this project and shall be as follows:

| Coating System | Coat | Product | DFT (Mini- mum) | Color |
|------------------|--------------------|--------------------------------|--------------------|------------|
| | 1st Coat | Carboguard 60 | 5-6 mils | Mill White |
| Carboline | 2nd Coat | Carboguard 60 | 5-6 mils | Beige |
| | Min DFT for System | | 10 mils | |
| | 1st Coat | Macropoxy 646 | 5-6 mils | Mill White |
| Sherwin Williams | 2nd Coat | Macropoxy 646 | 5-6 mils | Beige |
| | Min DFT for System | | 10 mils | |
| PPG | 1st Coat | Amercoat 278 Amercoat 2/400 | 5-6 mils | Mill White |
| | 2nd Coat | Amercoat 278 Amercoat 2/400 | 5-6 mils | Beige |
| | Min DFT for System | | 10 mils | |

D. Tank Exterior Disinfection Solution: A solution of 5.25% Sodium Hypochlorite, Biodegradable Soap and Water resulting in a solution of 200 ppm Sodium Hypochlorite.

PART 3 – EXECUTION

3.01 SURFACE PREPARATION

- A. <u>Rough Areas:</u>
 - 1. Burrs, sharp edges, corners, weld spatter, or rough welds which would cause difficulty in achieving a defect free coating shall be ground smooth to accept the coatings.
 - 2. Sharp edges and corners shall be ground round to accept coatings
- B. Surface Preparation:
 - 1. Prior to installation, all steel equipment railing, and anchor irons shall be abrasive blasted to an SSPC-SP6 Commercial Blast Cleaning Condition so as create an anchor profile of a minimum of 1.5 mils.
 - 2. All areas on the Tank Exterior that have rusting, peeling paint, lichen growth, areas where coating destructive testing was performed AND all areas where the equipment railing, and the angle irons have been welded on the Tank Exterior shall be cleaned to an SSPC-SP11 Power Tool Cleaning Condition. The existing anchor profile at the power tool cleaned surfaces is to be maintained and shall not be less than 1 mil. The outlying areas of coating where power tool cleaning was performed will need to be back to tightly adhered coating and feathered back so that a smooth transition in applied coating can be achieved.

- 3. Mold and lichen growth has been observed on the Tank Exterior. After welding on the tank roof and column leg, and power tool cleaning and epoxy mastic application has been completed, the tank should be sprayed with a Sodium Hypochlorite and Soap Solution as Specified in Part 2.01.D. The solution should be allowed to have a 20 to 30-minute contact time after which it should be pressure washed with clean water with pressures between 3,500 to 5,000 psi to remove all existing mold, lichen, dirt and any lose coating that may exist.
- 4. Additional Surface Preparation as Specified 09900 General Specifications for Coating Systems may be required.
- C. <u>Surface Contamination</u>:
 - 1. Remove all dust, moisture, mud, oil, grease, or other foreign material which would cause coating adhesion problems.
 - 2. If tests by the Field Inspector find questionable amounts of contamination on the steel substrates or painted substrates to be top-coated clean to remove contaminates.
 - 3. If questions or disagreements arise regarding the suitability of the substrate to receive a coating, secure the services of a representative of the coating manufacturer to examine the substrates in question and determine the suitability of the substrates to receive the coating. The manufacturer's written determination will be considered by the Owner/Engineer for making the final decision.

3.02 APPLICATION

A. Equipment Railing and Angle Irons:

- 1. After surface preparation of the equipment railing and the angle irons has been completed as Specified in Part 3.01.B.1, they shall be coated with two coats of an epoxy coating as Specified in Part 2.01.C.
- 2. The coating on the railing posts shall be held back 2 inches from the bottom of the posts to allow welding on the posts to the steel reinforcing pads on the tank roof. The coating shall also be held back from the edges leg of the angle irons that will be welded to the tank.
- B. Power tool cleaned surfaces:
 - 1. After surface preparation for the Tank Exterior has been completed as Specified in Part 3.01.A.1 and 3.01.B.3, and before the end of each day's work is finished, an epoxy mastic coat as Specified in Part 2.01.B shall be applied to the cleaned areas.
- C. Sealer Coat:
 - 1. After the Tank Exterior has been cleaned and pressure washed as Specified in Part 3.01.A.3, and the Tank is dry and clean, and spot priming has been completed as Specified in Part 2.01.B and 3.01.B.3, all the surfaces of the tank exterior including all welded equipment railings and angle irons as well as the Tank support legs shall be coated with a Penetrating Sealer as Specified in Part 2.01.B.

- D. Finish Coat:
 - 1. After Sealer Coat has been applied and dried, all surfaces of the tank exterior including all welded equipment railings and angle irons as well as the Tank support legs shall be coated with the finish coat as Specified Part 2.01.B.
 - 2. Any drips, runs, sags, micro bubbling, etc. from application of the Finish Coat shall be repaired per the Coating Manufacturer's recommendations to ensure a smooth finished surface. The Contractor shall also be responsible for removal of any debris or insects that may become stuck in the coating. Areas where debris and/or insects are removed from the finish coating shall be recoated.
 - 3. The finish coating shall be form one batch number for the entire application. If more than one batch number is used the entire tank shall be recoated with one batch number.
 - 4. Areas inside of the handrail boundary to be textured for slip resistance utilizing a sand aggregate. See contractor drawings for location.
- E. Coating Systems Labeling:
 - 1. Following the application of the final coat of the exterior finish color, label the tank to indicate the coating system used on the interior and exterior of the tank.
 - 2. Use a ¹/₂-inch tall lettering stencil with black paint and apply to the back side of the ladder vandal deterrent or other location as directed by the Owner.
 - 3. Include for each coat:
 - (i) The generic name of the coating.
 - (ii) The manufacturer product designation.
 - (iii) The average dry film thickness.
 - (iv) The manufacturer's color designation.

END OF SECTION

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| EQUIP EQUIPMENT ORG ORGINAL Weilded Wirke storp EQUIPMENT PLT PLATE WWF WATER STOP EW EACH WAY PLF POUNDS PER LINEAR FOOT WWF WATER STOP DESIGN CRITERIA A REFERENCES: 1. ICC INTERNATIONAL BUILDING CODE, 2015 EDITION RISK CATEGORY III IN ACCORDANCE WITH TABLE 1004.5 State Structures 3. AWWA D-100-11 B. DEAD LOADS: B. DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. PI = 5 PSF UNIND CADD MOV LOAD, PI = 5 PSF UNIND CADD MOV TANCE FACTOR, I = 1.0 SNOW EXPOSURE FACTOR, C = 1.0 SNOW EXPOSURE FACTOR, I = 1.1 E. WIND LOAD IPER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE FACTOR, I = 1.0 TOPOGRAPHY = 0.0 VELOCIT OVERTION, PR OVER DEATGORY = 0.0 VIND EXPOSURE CATEGORY = 0.0 VIND EXPOSURE CATEGORY = 0.0 VIND EXPOSURE CATEGORY = 0.0 VELOCIT OVERTION, PR NWAD AD 100 VERT, PROJ, PLANE) = 1.3 MIND EX | | | | | | |
| EQUIP EQUIPMENT PL PLATE WWF WELDED WIRE FABRIC EW EACH WAY PL POUNDS PER LINEAR FOOT WWF WELDED WIRE FABRIC DESIGN CRITERIA A. REFERENCES: 1. ICC INTERNATIONAL BUILDING CODE, 2015 EDITION RISK CATEGORY IIIN ACCORDANCE WITH TABLE 1804.5 2. ASCENSI 7-10 - MININUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES 3. AWWA D-100-11 B. DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) C. C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. = 5PSF SNOW LOAD IMPORTANCE FACTOR, I = 10 SNOW LOAD IMPORTANCE FACTOR, I E. WIND LOAD IPER AWWA D-100-11, 3.14; BASIC WIND SPEED, Y = 120 MPH WIND EXPOSURE FACTOR, CA E. WIND LOAD IPER AWWA D-100-11, 3.14; BASIC WIND SPEED, Y = 13 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) F. SEISMIC DESIGN DATA: = 1.3 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 1.3 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) F. SEISMIC DESIGN DATA: = 1.02 SIST = 0.00 SIST SEISMIC LIAPORTANCE FACTOR, I = 1.25 SIST = 0.00 SIST SIST = 0.02 SIST = 0.03 SIST SEISMIC LOAD DESIGN CATEGORY = 4.7 SIST SEISMIC LOAD DESIGN CATEGORY = 4.2 SIST < | EQ | EQUAL | ORIG | ORIGINAL | | |
| DESIGN CRITERIA A. REFERENCE: 1. SCENECTORY 2. SCENECTORY 3. SCENECTORY < | | | | | WWF | WELDED WIRE FABRIC |
| A. REFERENCES: 1. ICC INTERNATIONAL BUILDING CODE, 2015 EDITION RISK CATEGORY III IN ACCORDANCE WITH TABLE 1604.5 2. ASOCISIT-710 - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES 3. AWWA D-100-11 B. DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. Pg = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW LOAD IMPORTANCE FACTOR, I = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, CC = 1.0 SNOW LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED. V = 100 FORCE COEFFICIENT, CC = 0.6 VELOCITY COEFFICIENT, K2 = 0.8 MEMORITANCE FACTOR, I = 1.15 WIND PRESURE (APPLIED TO VERT. PROJ. PLANE) = 4.15 WIND PRESURE (APPLIED TO VERT. PROJ. PLANE) = 4.15 SSI = 0.023 SD1 = 0.033 STI C LASS = 0.033 STI C LASS = 0.033 STI C LASS = 0.049 SEISMIC LOAD ESIGN CATEGORY = 74 SEISMIC LOAD ESIGN CATEGORY = 74 SEISMIC DESIGN CATEGORY = 1.25 SSI = 0.049 SEISMIC DAST AND FACTOR, I = 1.25 SSI = 0.033 SD1 = 0.033 STI C LASS = 0.033 SD1 = 0.033 STI C LASS = 0.033 SD1 = 0.037 STI C LASS = 0.049 SEISMIC LOAD ESIGN CATEGORY = 74 SEISMIC LOAD ESIGN CATEGORY = 74 SEISMIC LOAD ESIGN DATA: SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 5.0.049 & Ss 5.0.159 DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | | · <u>-</u> · | | | |
| LICC INTERNATIONAL BUILDING CODE, 2015 EDITION RISK CATEGORY III IN ACCORDANCE WITH TABLE 1604.5 ACKESEI 7-10- MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES AWWA D-100-11 DEAD LOADS: ROOF DEAD LOAD (SELF WEIGHT) LIVE LOADS (U.N.O.): ROOF ROOF SNOW LOAD. B. OFOF SNOW LOAD. GROUND SNOW LOAD, Pg SNOW EXPOSURE ACTOR, Ce UNFORM ROOF DESIGN SNOW LOAD, Pf SNOW EXPOSURE FACTOR, Ce INFORM ROOF DESIGN SNOW LOAD, Pf SNOW LOAD IMPORTANCE FACTOR, I INFORMAL FACTOR, CQ INFORMAL FACTOR, CG SNOW EXPOSURE FACTOR, CE WIND LOAD IPER AWWA D-100-11, 31.4!: BASIC WIND SPEED, Y SNOW EXPOSURE FACTOR, I INFORMAL FACTOR, CA INFORTANCE FACTOR, I INFORTANCE FACTOR, I | | | DESIG | N CRITERIA | | |
| RISK CATEGORY III IN ACCORDANCE WITH TABLE 1604.5 2. ASCECSEI 7-10- MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES 3. AWWA D-100-11 B. DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. D. ROOF SNOW LOAD. GROUND SNOW LOAD. Pg = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW EVPOSURE FACTOR, C = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, C = 1.1 E. WIND LOAD JPER AWWA D-100-11, 31.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COERPHY = 1.15 WIND DEAD DATA: F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC DESIGN DESIGN SNOW LOAD, PI = 1.25 S1 = 0.023 S01 = 0.023 S01 = 0.023 S01 = 0.033 S11 CLOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 s 0.049 & Ss s 0.159 DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | Α. | | | | | |
| 3. AWWA D-100-11 DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) ROOF SNOW LOAD. Pg = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW LOAD UNIFORMANCE FACTOR, C = 10 SNOW LOAD IMPORTANCE FACTOR, C = 1,1 = 1,1 THERMAL FACTOR, CC = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: = BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: = BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: = BASIC WIND SPEED. V = 120 MPH WIND LOAD [PER AWWA D-100-11, 3.1.4]: = BASIC WIND SPEED. V = 120 MPH WIND PORTANCE FACTOR, I = 1,1 = 1,1 = 1,0 = 1,0 = 1,0 = 1,0 = 0,05 = 0,05 SI WIND PRESSURE (APPLED TO VERT. PROJ. PLANE) = 4,3 PSF = 1,15 WIND PRESSURE (APPLED TO VERT. PROJ. PLANE) = 4,3 PSF = 1,15 = 0,05 SI SI SI | | RISK CATEGORY | III IN ACCORDANCE WI | TH TABLE 1604.5 | | |
| B. DEAD LOADS: ROOF DEAD LOAD = (SELF WEIGHT) C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. PG B. SNOW LOAD. PS B. SNOW LOAD. PG | | | /INIMUM DESIGN LOADS | S FOR BUILDINGS AND OTHER STRU | CTURES | |
| ROOF DEAD LOAD = (SELF WEIGHT) C. LIVE LOADS (U.N.C.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. Pg = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW EXPOSUBLE FACTOR, C0 C. WIND LOAD [PER AWWA D-100-11, 3.1.4]: = 1.0 SNOW EXPOSUBLE CATEGORY = 1.0 DIRECTIONALITY FACTOR, Kd E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: = 1.0 FORCE COEFFICIENT, Kz = 1.0 SNOW EXPOSUBLE CATEGORY FORCE COEFFICIENT, Kz = 1.0 WIND PRESSURE (ATEGORY = 1.0 FORCE COEFFICIENT, Kz = 1.0 SEISMIC DESIGN DATA: F. SEISMIC DESIGN DATA: = 1.25 SS = 0.05 SS = 0.05 SS SDS = 0.05S SS = 0.05S SS< | 5 | | | | | |
| C. LIVE LOADS (U.N.O.): ROOF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD: $\begin{cases} GROUND SNOW LOAD, Pg = 5 PSF \\ UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF \\ SNOW EXPOSURE FACTOR, Ce = 1.0 \\ SNOW LOAD IMPORTANCE FACTOR, Ce = 1.1 \\ THERMAL FACTOR Ct = 1.1 \\ E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC DESIGN CATEGORY = 0.053 SDS = 0.053 SDS$ | В. | DEAD LOADS: | | | | |
| ROF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. GROUND SNOW LOAD, Pg = 5 PSF SNOW LOAD IMPORTANCE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, Ci = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.3 MIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DEFICIENT, Kz = 1.25 SS = 0.053 SDS = 0.053 SDS = 0.053 SDS = 0.053 SDS = 0.037 STE CLASS = 0.033 STE CLASS = 0.034 STE SIMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE | | ROOF DEAD LOAD | | = (SELF WEIGHT) | | |
| ROF = 20 PSF (OR 300 LB POINT LOAD) D. ROOF SNOW LOAD. GROUND SNOW LOAD, Pg = 5 PSF SNOW LOAD IMPORTANCE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, Ci = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.3 MIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DEFICIENT, Kz = 1.25 SS = 0.053 SDS = 0.0337 SDS = 0.0337 SDS = 0.033 SDS = 0.042 BURC LOADES SEISMIC DESIGN CATEGORY = 'A' SEISMIC DATA: = 0.25 SDS = 0.0337 SDS = 0.0337 SDS = 0.0337 SDS = 0.042 BURC | C. | LIVE LOADS (U.N.O.): | | | | |
| D. ROOF SNOW LOAD: $\begin{cases} GROUND SNOW LOAD, Pg = 5 PSF \\ UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF \\ SNOW EXPOSURE FACTOR, Ce = 1.0 \\ SNOW LOAD IMPORTANCE FACTOR, I = 1.1 \\ THERMAL FACTOR, Ct = 1.1 \end{cases}$ E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Cf = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.023 SDS = 0.023 SDS = 0.033 SITE CLASS = 10' SEISMIC DESIGN CATEGORY = 'A' SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 < 0.04g & 8s < 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | | | | | |
| GROUND SNOW LOAD, Pg = 5 PSF UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, Ct = 1.1 THERMAL FACTOR, Ct = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.023 SDS = 0.023 SDS = 0.023 SDS = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | | RUUF | | - 20 PSF (UK 300 LB PUIN | II LUAD) | |
| UNIFORM ROOF DESIGN SNOW LOAD, Pf = 5 PSF SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, Ct = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.053 SD1 = 0.037 SITE CLASS = D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | D. | | | | | |
| SNOW EXPOSURE FACTOR, Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR, I = 1.1 THERMAL FACTOR, Ct = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SEVESURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.037 SITE CLASS = 10' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | | | | - | | |
| THERMAL FACTOR, Ct = 1.1 E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.055 S1 = 0.023 SD1 = 0.023 SD1 = 0.037 SITE CLASS = 0.037 SITE CLASS = 0.049 SEISMIC DESIGN CATEGORY = 'A' SEISMIC IDAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 < 0.049 & Ss < 0.159 DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | SNOW EXPOSURE F | FACTOR, Ce | = 1.0 | | |
| E. WIND LOAD [PER AWWA D-100-11, 3.1.4]: BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Kz = 0.6 VELOCITY COEFFICIENT, Kz = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.005 S1 = 0.023 SDS = 0.053 SD1 = 0.023 SD1 = 0.037 SITE CLASS = D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC IDAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 < 0.04g & Ss < 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | SNOW LOAD IMPOR | RTANCE FACTOR, I | = 1.1 | | |
| BASIC WIND SPEED, V = 120 MPH WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.023 SDS = 0.053 SD1 = 0.037 SITE CLASS = D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | | | | - 1.1 | | |
| WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.023 SDS = 0.053 SD1 = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | | | 100-11 3 1 41 | | | |
| WIND EXPOSURE CATEGORY = C DIRECTIONALITY FACTOR, Kd = 0.85 TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.023 SDS = 0.053 SD1 = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DESIGN CATEGORY = 'A' | E. | | 100-11, 0.1.4]. | | | |
| TOPOGRAPHY = 1.0 FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SD = 0.053 SD = 0.053 SD = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 \leq 0.04g & Ss \leq 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | E. | WIND LOAD [PER AWWA D- | - | = 120 MPH | | |
| FORCE COEFFICIENT, Cf = 0.6 VELOCITY COEFFICIENT, Kz = 1.38 IMPORTANCE FACTOR, I = 1.15 WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.053 SD1 = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC DOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 \leq 0.04g & Ss \leq 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | E. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C |), V ATEGORY | = C | | |
| $\begin{tabular}{lllllllllllllllllllllllllllllllllll$ | E. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY |), V ATEGORY ACTOR, Kd | = C = 0.85 = 1.0 | | |
| WIND PRESSURE (APPLIED TO VERT. PROJ. PLANE) = 43 PSF F. SEISMIC DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I = 1.25 SS = 0.05 S1 = 0.023 SDS = 0.037 SITE CLASS = 'D' SEISMIC DESIGN CATEGORY = 'A' SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 ≤ 0.04g & Ss ≤ 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | E. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN |), V ATEGORY ACTOR, Kd NT, Cf | = C = 0.85 = 1.0 = 0.6 | | |
| SEISMIC IMPORTANCE FACTOR, I= 1.25SS= 0.05S1= 0.023SDS= 0.053SD1= 0.037SITE CLASS= 'D'SEISMIC DESIGN CATEGORY= 'A'SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATEDWHERE S1 $\leq 0.04g$ & Ss $\leq 0.15g$ DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | E. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz 'OR, I | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 | | |
| $\begin{array}{llllllllllllllllllllllllllllllllllll$ | E. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz 'OR, I | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 | | |
| $\begin{array}{llllllllllllllllllllllllllllllllllll$ | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz 'OR, I | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 | | |
| SDS= 0.053 SD1= 0.037 SITE CLASS= 'D'SEISMIC DESIGN CATEGORY= 'A'SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATEDWHERE S1 ≤ $0.04g$ & Ss ≤ $0.15g$ DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz TOR, I APPLIED TO VERT. PROJ | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF | | |
| $ \begin{array}{llllllllllllllllllllllllllllllllllll$ | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC IMPORTAN SS |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz TOR, I APPLIED TO VERT. PROJ | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 | | |
| SEISMIC DESIGN CATEGORY = 'A' SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 \leq 0.04g & Ss \leq 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC IMPORTAN SS S1 |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz TOR, I APPLIED TO VERT. PROJ | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 | | |
| SEISMIC LOAD EXEMPTION - PER AWWA D-100-11, STRUCTURES LOCATED WHERE S1 \leq 0.04g & Ss \leq 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS SD1 |), V ATEGORY ACTOR, Kd NT, Cf IENT, Kz TOR, I APPLIED TO VERT. PROJ | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 | | |
| WHERE S1 \leq 0.04g & Ss \leq 0.15g DO NOT REQUIRE DESIGN FOR SEISMIC LOADS. | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS SD1 SITE CLASS | 9, V ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 = 'D' | | |
| | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS SD1 SITE CLASS SEISMIC DESIGN CA | ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 = 'D' = 'A' | | |
| G. ICE LOAD DATA: | | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS S1 SDS S1 SITE CLASS SEISMIC DESIGN CA SEISMIC DESIGN CA | 9, V ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I ATEGORY MPTION - PER AWWA D- | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 = 'D' = 'A' -100-11, STRUCTURES LOCATED | | |
| | F. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS SD1 SITE CLASS SEISMIC DESIGN CA SEISMIC DESIGN CA SEISMIC LOAD EXEN WHERE S1 ≤ 0.04g 8 | 9, V ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I ATEGORY MPTION - PER AWWA D- | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 = 'D' = 'A' -100-11, STRUCTURES LOCATED | | |
| ICE THICKNESS = 0.5 IN ICE WIND SPEED = 30 MPH | F. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC DESIGN DATA: SS S1 SDS S1 SITE CLASS SEISMIC DESIGN CA SEISMIC DESIGN CA SEISMIC LOAD EXEI WHERE S1 ≤ 0.04g 8 | 9, V ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I ATEGORY MPTION - PER AWWA D- | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.053 = 0.037 = 'D' = 'A' -100-11, STRUCTURES LOCATED QUIRE DESIGN FOR SEISMIC LOADS. | | |
| | F. | WIND LOAD [PER AWWA D- BASIC WIND SPEED WIND EXPOSURE C DIRECTIONALITY FA TOPOGRAPHY FORCE COEFFICIEN VELOCITY COEFFIC IMPORTANCE FACT WIND PRESSURE (A SEISMIC DESIGN DATA: SEISMIC IMPORTAN SS S1 SDS SD1 SITE CLASS SEISMIC DESIGN CA SEISMIC DESIGN CA SEISMIC LOAD EXEN WHERE S1 ≤ 0.04g & ICE LOAD DATA: ICE THICKNESS | 9, V ATEGORY ACTOR, Kd NT, Cf IENT, Kz OR, I APPLIED TO VERT. PROJ ICE FACTOR, I ATEGORY MPTION - PER AWWA D- | = C = 0.85 = 1.0 = 0.6 = 1.38 = 1.15 J. PLANE) = 43 PSF = 1.25 = 0.05 = 0.023 = 0.023 = 0.037 = 'D' = 'A' -100-11, STRUCTURES LOCATED QUIRE DESIGN FOR SEISMIC LOADS. = 0.5 IN | | |

| | SPECIAL INSPECTION REQUIREMENTS FOR STEEL CONSTRUCTION (STRUCTURAL STEEL): | | | 3 5 |
|--|--|--|---|---|
| A. REFERENCES:1. AISC STEEL CONSTRUCTION MANUAL, 14TH EDITION | R - INSPECT THESE ITEMS ON A <u>RANDOM</u> BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS | | | NO. F-392 tech.cor |
| 2. AWS D1.1 STRUCTURAL WELDING CODE - STEEL | C - INSPECT THESE ITEMS ON A <u>CONTINUOUS</u> BASIS | | | E REG N |
| B. MATERIALS: | | С | R N/A | |
| 1. GRADE STEEL | | v | | |
| CHANNELS, ANGLES, AND PLATESASTM A36 STRUCTURAL PIPE ASTM A53, GRADE B, EV=35, KSI | | × | | |
| | | X | | |
| 2. EXISTING GRADE STEEL: TANK PLATESA7-61T, Fy=33KSI (ASSUMED) | | - | × - | |
| 3. STRUCTURAL BOLTS: ASTM A325-N | | - | × - | |
| 4. WELDS: E70XX ELECTRODES C. CONNECTIONS 1. THE ASSEMBLY SURFACE, INCLUDING THOSE ADJACENT TO THE WASHER, SHALL BE FREE OF MILL SCALE, | - JOINT PREPARATION. - DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL). - CLEANLINESS (CONDITION OF STEEL SURFACES). - TACKING (TACK WELD QUALITY AND LOCATION). | - | x - | |
| RE FOOT 2. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A BOLT TENSION NOT LESS THAN THAT | | | X - | TATE OF TE |
| RE INCH SPECIFICATION IN THE AISC MANUAL. FULL TENSIONING SHALL BE BY THE TURN OF NUT METHOD, BY A DIRECT TENSION INDICATOR, OR BY PROPERLY CALIBRATED WRENCHES. PROVIDE HARDENED WASHERS | | - | | STALL OF |
| UNDER THE NUT OR BOLT HEAD, WHICHEVER IS THE ELEMENT TURNED IN TIGHTENING. 3. WELDING - PERFORM ALL WELDING IN ACCORDANCE WITH AWS D1.1 CODE, LATEST EDITION, WELDS SHALL BE MADE ONLY BY OPERATORS CERTIFIED BY AWS IN PERFORMING THE TYPE OF WORK INDICATED. | G. FIT-UP OF FILLET WELDS. - DIMENSIONS (ALIGNMENT, GAPS AT ROOT). - CLEANLINESS (CONDITION OF STEEL SURFACES). - TACKING (TACK WELD QUALITY AND LOCATION) | - | X - | PHILLIP A. FLEM |
| D. TOLERANCES: AISC CODE OF STANDARD PRACTICE (LATEST EDITION) | 2. INSPECTION TASKS DURING WELDING | I | | |
| E. SHOP DRAWINGS | A. USE OF QUALIFIED WELDERS | - | X - | 8-33-19 |
| SUBMIT ERECTION AND FABRICATION SHOP DRAWINGS. SEE SPECS. SUBMIT ERECTION PROCEDURES AND TEMPORARY BRACING PLAN FOR ENGINEER REVIEW. | B. CONTROL AND HANDLING OF WELDING CONSUMABLES - PACKAGING - EXPOSURE CONTROL | - | x - | U-27-1-1 |
| ALL SURFACES, EXCEPT THOSE INTENDED TO RECEIVE FIELD WELDING, SLIP CRITICAL BOLTS, OR | C. NO WELDING OVER CRACKED TACK WELDS | - | X - | |
| | D. ENVIRONMENTAL CONDITIONS - WIND SPEED WITHIN LIMITS - PRECIPITATION AND TEMPERATURE | - | x - | DNIO ER |
| SPECIAL INSPECTION REQUIREMENTS | | | | N VTON ATEF |
| SPECIAL INSPECTIONS REQUIRED FOR THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE | - SETTINGS ON WELDING EQUIPMENT - TRAVEL SPEED | | | SAN ANT WAT |
| WITH THE 2015INTERNATIONAL BUILDING CODE. SPECIAL INSPECTIONS SHALL BE PERFORMED BY AN APPROVED INSPECTION AGENCY U.N.O, EMPLOYED BY THE OWNER. | - SELECTED WELDING MATERIALS - SHIELDING GAS TYPE/FLOW RATE | - | X - | |
| THE SPECIAL INSPECTOR SHALL BE CERTIFIED BY THE INTERNATIONAL CODE COUNCIL (I.C.C.) TO PERFORM INSPECTION FOR THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. | - PREHEAT APPLIED - INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.) - PROPER POSITION (F, V, H, OH) | | | |
| THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND/OR THE ENGINEER. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE STRUCTURAL ENGINEER AND TO THE BUILDING OFFICIAL. | F. WELDING TECHNIQUES - INTERPASS AND FINAL CLEANING - EACH PASS WITHIN PROFILE LIMITATIONS - EACH PASS MEETS QUALITY REQUIREMENTS | - | x - | |
| THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK | 3. INSPECTION TASKS AFTER WELDING | | | |
| CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE | A. WELDS CLEANED | - | X - | BY |
| | B. SIZE, LENGTH AND LOCATION OF WELDS | X | | |
| ADVANCE NOTICE TO THE OWNER/OWNER'S REPRESENTATIVE WHEN HIS WORK IS READY FOR | C. WELDS MEET VISUAL ACCEPTANCE CRITERIA - CRACK PROHIBITION | | | |
| | - CRATER CROSS SECTION | x | - - | |
| THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK | - WELD SIZE | | | |
| FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PERIODIC | - UNDERCUT - POROSITY | | | |
| COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN | D. ARC STRIKES | X | | |
| ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. | E. K-AREA WHEN WELDING OF DOUBLED DLATES, CONTINUUTY DLATES OD STIEFENEDS | | | RIPTION VDUM 2 |
| | HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA | x | - - | ENDU |
| OWNER OR OWNER'S REPRESENTATIVE SHALL BE SYNONYMOUS WITH 'BUILDING OFFICIAL' IN THE FOREGOING IF THE PROJECT IS NOT UNDER THE JURISDICTION OF A BUILDING DEPARTMENT. | F. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED) | x | | ADD |
| SPECIAL INSPECTION SHALL BE PROVIDED FOR THE FOLLOWING TYPES OF WORK PERFORMED IN | G. REPAIR ACTIVITIES | X | | 3/19 3/19 |
| THE FIELD, OR NOT PERFORMED IN AN APPROVED FABRICATION SHOP AS DEFINED ABOVE, UNLESS NOTED AS "N/A". | H. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER | X | | DAT 8/23/ |
| ER SQUAF | CHANNELS, ANGLES, AND FLATES | Note:::::::::::::::::::::::::::::::::::: | - Doubling - Doubl | Image: Second |

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PROJ: 200-09308-19001-01

S-001

PAF

SPS

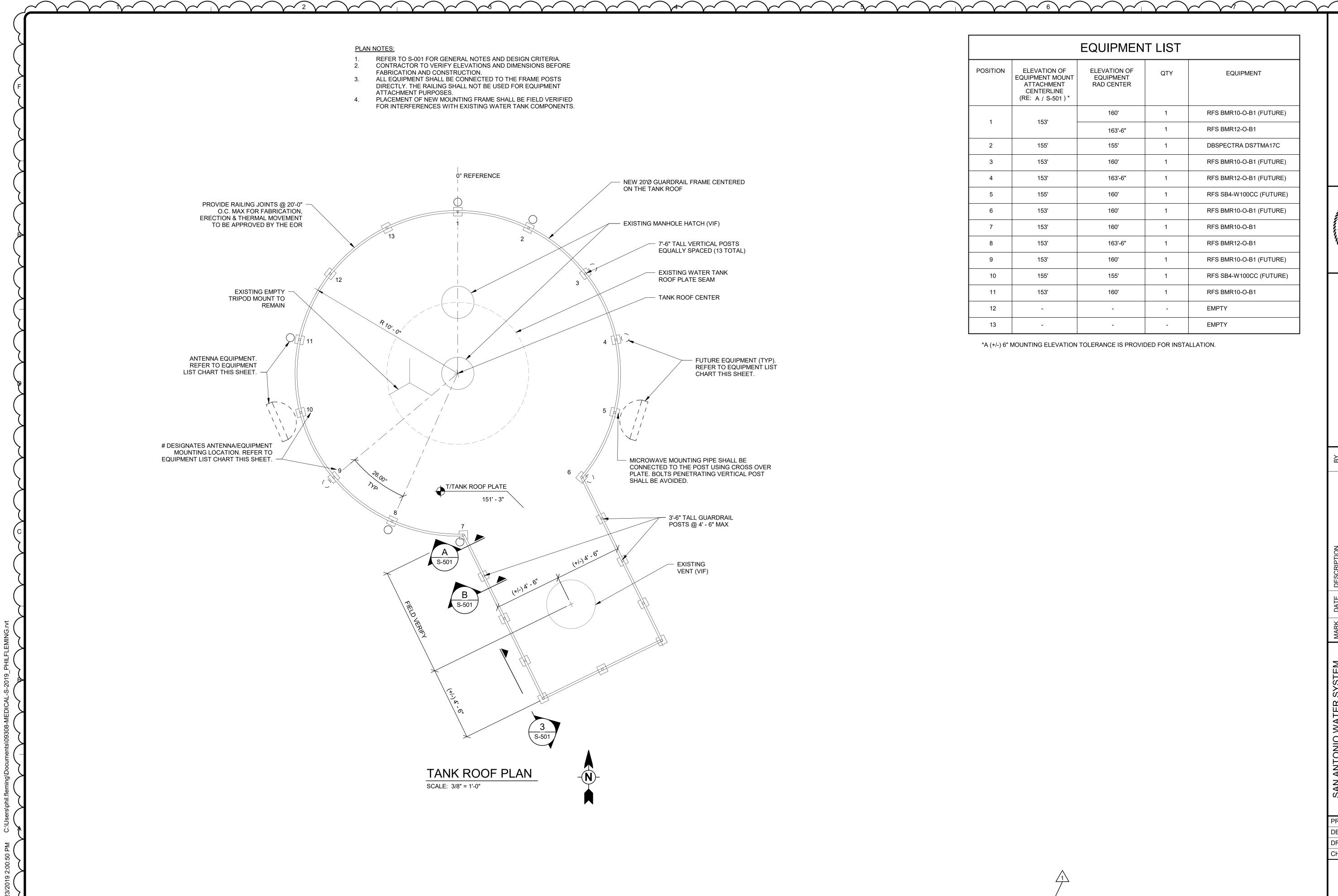
JLB

DESN:

DRWN:

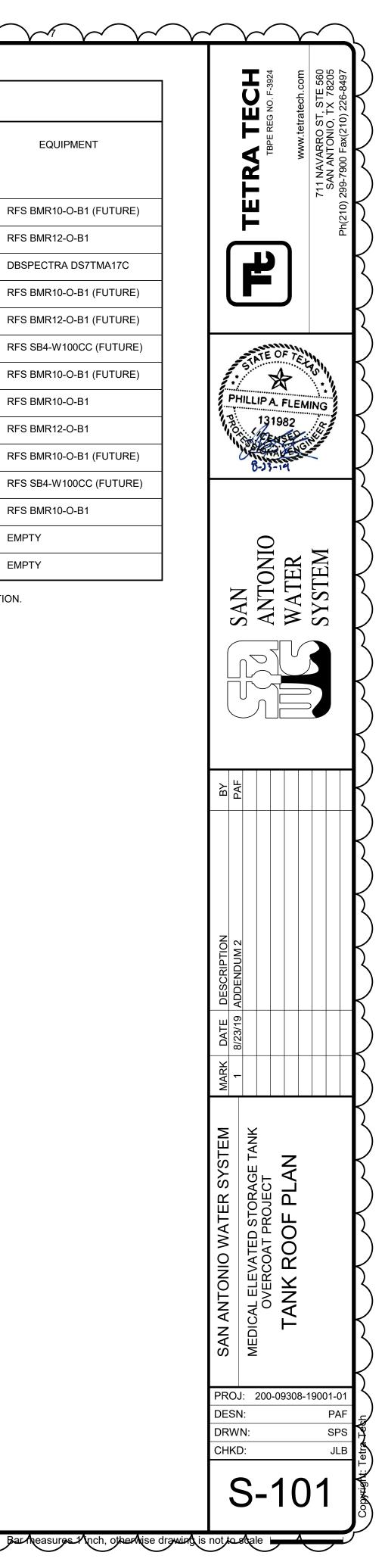
CHKD:

Bartheasures Minch, otherwise drawing is not to scale



| EQUIPMENT LIST | | | | | |
|----------------|---|---|-----|-------------------------|--|
| POSITION | ELEVATION OF EQUIPMENT MOUNT ATTACHMENT CENTERLINE (RE: A / S-501)* | ELEVATION OF EQUIPMENT RAD CENTER | QTY | EQUIPMENT | |
| 1 | 153' | 160' | 1 | RFS BMR10-O-B1 (FUTURE) | |
| I | 100 | 163'-6" | 1 | RFS BMR12-O-B1 | |
| 2 | 155' | 155' | 1 | DBSPECTRA DS7TMA17C | |
| 3 | 153' | 160' | 1 | RFS BMR10-O-B1 (FUTURE) | |
| 4 | 153' | 163'-6" | 1 | RFS BMR12-O-B1 (FUTURE) | |
| 5 | 155' | 160' | 1 | RFS SB4-W100CC (FUTURE) | |
| 6 | 153' | 160' | 1 | RFS BMR10-O-B1 (FUTURE) | |
| 7 | 153' | 160' | 1 | RFS BMR10-O-B1 | |
| 8 | 153' | 163'-6" | 1 | RFS BMR12-O-B1 | |
| 9 | 153' | 160' | 1 | RFS BMR10-O-B1 (FUTURE) | |
| 10 | 155' | 155' | 1 | RFS SB4-W100CC (FUTURE) | |
| 11 | 153' | 160' | 1 | RFS BMR10-O-B1 | |
| 12 | - | - | - | EMPTY | |
| 13 | - | - | - | EMPTY | |

*A (+/-) 6" MOUNTING ELEVATION TOLERANCE IS PROVIDED FOR INSTALLATION.



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